

AD-A131 912

INTERACTION OF A DISLOCATION WITH A CRACK(U) PRINCETON  
UNIV NJ DEPT OF CIVIL ENGINEERING A C ERINGEN AUG 83  
RR-83-SM-9 N00014-76-C-0240

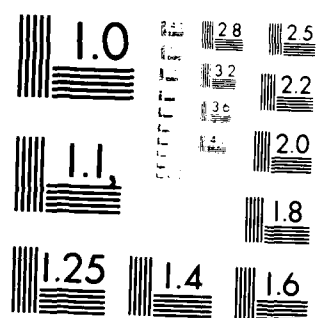
1/1

UNCLASSIFIED

F/G 12/1

NL

END
DATE
FILMED
9- 83
DTIC



MICROCOPY RESOLUTION TEST CHART  
 NATIONAL BUREAU OF STANDARDS-1963-A

ADA 81912

INTERACTION OF A DISLOCATION WITH A CRACK

A. Cemal Eringen  
PRINCETON UNIVERSITY

Technical Report No. 59  
Civil Engng. Res. Rep. No. 83-SM-9

AUG 30 1983

Research Sponsored by the  
OFFICE OF NAVAL RESEARCH  
under  
Contract N00014-76-C-0240 Mod 4  
Task No. NR 064-410

Aug. 1983

Approved for public release:  
distribution unlimited

Reproduction in whole or in part is permitted  
for any purpose of the United States Government

DTIC FILE COPY

83 08 26 064



## INTERACTION OF A DISLOCATION WITH A CRACK

A. Cemal Eringen  
Princeton University  
Princeton, NJ 08544

Distribution/	
Availability of this	
Availability of	
Dist	Special
H	

### ABSTRACT

A solution is given of the field equations of nonlocal elasticity for a line crack interacting with a screw dislocation in an elastic plane under anti-plane shear loading. Displacement and stress fields are determined throughout the core region and beyond. In the case when the dislocation is absent, the circumferential stress is shown to vanish at the crack tip, increasing to a maximum along the crack line afterwards decreasing to its classical value at large distances from the crack tip. This is in contradiction with the classical elasticity solutions which predicts stress singularity at the crack tip and it is in accordance with the physical condition that the crack tip surface must be free of surface tractions. The presence of the dislocation alters the stress distribution considerably when it is close to the crack tip. The stress distributions, in the core region, are displayed. A fracture criterion based on the maximum stress is established and used to determine the theoretical strengths of pure crystals that contain a line crack. Results are in good agreement with those based on the atomic theories and experiments.

SECURITY CLASSIFICATION OF THIS PAGE (When Data Entered):

REPORT DOCUMENTATION PAGE		READ INSTRUCTIONS BEFORE COMPLETING FORM
1 REPORT NUMBER PRINCETON UNIV. TECHNICAL REPORT #59	2 GOVT ACCESSION NO.	3 RECIPIENT'S CATALOG NUMBER
4 TITLE (and Subtitle) INTERACTION OF A DISLOCATION WITH A CRACK	5 TYPE OF REPORT & PERIOD COVERED Technical Report	
	6 PERFORMING ORG. REPORT NUMBER 83-SM-9	
7 AUTHOR(s) A. Cemal Eringen	8 CONTRACT OR GRANT NUMBER(s) N00014-76-002040 Mod. 4	
9 PERFORMING ORGANIZATION NAME AND ADDRESS PRINCETON UNIVERSITY Princeton, NJ 08544	10 PROGRAM ELEMENT PROJECT TASK AREA & WORK UNIT NUMBERS NR 064-410	
11 CONTROLLING OFFICE NAME AND ADDRESS OFFICE OF NAVAL RESEARCH (Code 471) Arlington, VA 22217	12 REPORT DATE Aug. 1983	
	13 NUMBER OF PAGES 30	
14 MONITORING AGENCY NAME & ADDRESS (if different from Controlling Office)	15 SECURITY CLASS (of this report) unclassified	
	15a DECLASSIFICATION DOWNGRADING SCHEDULE	
16 DISTRIBUTION STATEMENT (of this Report)		
17 DISTRIBUTION STATEMENT (of the abstract entered in Block 20, if different from Report)		
18 SUPPLEMENTARY NOTES		
19 KEY WORDS (Continue on reverse side if necessary and identify by block number) dislocation, crack, fracture, interaction of dislocation and crack, nonlocal elasticity		
20 ABSTRACT (Continue on reverse side if necessary and identify by block number) A solution is given of the field equations of nonlocal elasti- city for a line crack interacting with a screw dislocation in an elastic plane under anti-plane shear loading. Displacement and stress fields are determined throughout the core region and beyond. In the case when the dislocation is absent, the circum- ferential stress is shown to vanish at the crack tip, increas- ing to a maximum along the crack line afterwards decreasing		

DD FORM 1 JAN 73 1473 EDITION OF 1 NOV 63 IS OBSOLETE

(cont)

SECURITY CLASSIFICATION OF THIS PAGE (When Data Entered):

ABSTRACT (cont)

to its classical value at large distances from the crack tip. This is in contradiction with the classical elasticity solutions which predicts stress singularity at the crack tip and it is in accordance with the physical condition that the crack tip surface must be free of surface tractions. The presence of the dislocation alters the stress distribution considerably when it is close to the crack tip. The stress distributions, in the core region, are displayed. A fracture criterion based on the maximum stress is established and used to determine the theoretical strengths of pure crystals that contain a line crack. Results are in good agreement with those based on the atomic theories and experiments.

## I. INTRODUCTION

It is well known that the classical elasticity solution of crack problems fail in a core region around a sharp crack tip, since they predict stress singularity at the tip. The assessment of the core radius and the stress field within the core is a problem usually discussed within the context of atomic theories of lattices (cf. [1]), even at that its treatment contains various assumptions regarding the interatomic arrangements and force fields.

Engineering fracture mechanics, on the other hand, is based on the Griffith's<sup>2</sup> ideas which resort to other concepts (e.g. energy, J-integral, fracture toughness). To be sure, there exist certain *ersatz* to account for the effect of the core region on fracture process in phenomenological ways. These are useful for engineering purposes, however, they are not based on a fundamental theory nor are they capable predicting the state of stress in the core region which is fundamental to the initiation of fracture.

In several previous papers, we have shown that nonlocal elasticity solutions of Griffith crack problems lead to finite stress at the crack tip<sup>3-5</sup>. In fact, an exact solution<sup>6</sup> obtained for the screw dislocation, indicates that the stress vanishes at the tip of the crack, growing to a maximum in the vicinity of the crack tip. The important implications of this result in connection with the initiation of fracture is the motivation for the present work.

The solution obtained here for the Mode III (anti-plane shear) problem for a crack interacting with a screw dislocation indicates that the circumferential stress field is vanishingly small (zero when the dislocation

is absent) at the crack tip, when the screw is located far away from the crack tip. When the dislocation is near the crack tip, the stress field is affected appreciably displaying several maxima near the crack tip. By equating the maximum stress to the cohesive yield stress, we can determine the stress intensity factor  $K_g$  or the theoretical yield stress, given  $K_g$ . Calculated  $K_g$  values, on the basis of the present theory, are in fair agreement with those determined experimentally. Theoretical strengths are also estimated by means of the dislocation model. Results agree with those predicted by atomic models.

The mathematical model of approach to the solution of this problem is new and possesses potential applications in other areas.

## 2. BASIC EQUATIONS

In several previous papers, we developed a theory of nonlocal elasticity, cf. [7, 8, 9]. For homogeneous and isotropic elastic solids, linear theory is expressed by the set of equations

$$(2.1) \quad t_{kl,k} + \rho(f_l - \ddot{u}_l) = 0,$$

$$(2.2) \quad t_{kl}(\underline{x}, t) = \int_V \alpha(|\underline{x}' - \underline{x}|, \tau) \sigma_{kl}(\underline{x}', t) dv(\underline{x}') ,$$

$$(2.3) \quad \sigma_{kl}(\underline{x}', t) = \lambda e_{rr}(\underline{x}', t) \delta_{kl} + 2 \mu e_{kl}(\underline{x}', t) ,$$

$$(2.4) \quad e_{kl}(\underline{x}', t) = \frac{1}{2} \left[ \frac{\partial u_k(\underline{x}', t)}{\partial x'_l} + \frac{\partial u_l(\underline{x}', t)}{\partial x'_k} \right]$$



where  $t_{kl}$ ,  $\rho$ ,  $f_l$  and  $u_l$  are respectively, the stress tensor, mass density, body force density and the displacement vector.  $\lambda$  and  $\mu$  are the Lamé elastic constants and  $\alpha$  is the "attenuation function" which depends on the distance  $|\underline{x}' - \underline{x}|$  and a parameter  $\tau$  which denotes the ratio of the internal characteristic length  $a$  to the external characteristic length  $\ell$ , i.e.

$$(2.5) \quad \tau = e_0 a / \ell$$

where  $e_0$  is a constant appropriate to each material. Characteristic lengths may be selected according to the range and sensitivity of the physical phenomena to be investigated. For instance, for perfect crystals,  $a$  may be taken as the lattice parameter and  $\ell$  as the half crack length. For granular materials,  $a$  may be considered to be the average granular distance and for fiber composites, the fiber distance etc. The material constant,  $e_0$  may be determined by one experiment.

Equations (2.1), (2.3) and (2.4) are those known from the theory of classical elasticity, but Eq. (2.2) is new, replacing Hooke's law. According to Eq. (2.2), the stress at a point  $\underline{x}$  depends on strains at all points  $\underline{x}'$  of the body. The attenuation function  $\alpha$  determines the degree of influence with the distance. From the physical nature of solids, it is clear that the influence of strains at  $\underline{x}'$ , on the stress at  $\underline{x}$ , decreases with the distance  $|\underline{x}' - \underline{x}|$ . Thus,  $\alpha(|\underline{x}' - \underline{x}|)$  must acquire its maximum at  $\underline{x}' = \underline{x}$ . Moreover, when  $a \rightarrow 0$ ,  $\alpha$  must become a Dirac delta measure so that nonlocal theory shall revert to classical elasticity theory. By matching the phonon dispersion curves with those resulting from nonlocal theory, we have determined  $\alpha$  for various cases (cf. [5], [8], [10]).

By discretizing Eq. (2.2), it can be shown that equations of nonlocal elasticity revert to those of atomic lattice dynamics.<sup>11</sup> Thus, it is clear that nonlocal theory is a suitable model for the treatment of physical phenomena with characteristic lengths in the range from the molecular or atomic dimensions to macroscopic sizes.

For a two-dimensional perfect lattice, the dispersion curves are matched in the entire Brillouin zone to within an error less than 1½% with the attenuation function<sup>10</sup>

$$(2.6) \quad \alpha(|\underline{x}|, \tau) = (2\pi\ell^2\tau^2)^{-1} K_0(\sqrt{\underline{x} \cdot \underline{x}}/\ell\tau)$$

where  $K_0$  is the modified Bessel's function. We note that Eq. (2.6) is Green's function for the operator  $L \equiv (1 - \ell^2\tau^2\nabla^2)$ , i.e.

$$(2.7) \quad (1 - \ell^2\tau^2\nabla^2)\alpha = \delta(|\underline{x}' - \underline{x}|)$$

In fact, it is possible to employ other linear operators to characterize the nature of nonlocal attractions of material points in solids. This apparent non-uniqueness of  $\alpha$  may be considered to be a defect of the theory. On the contrary, for imperfect and amorphous solids, this may provide a desirable flexibility. Ultimately, however,  $\alpha$  should be determined from experimental and/or statistical mechanical considerations. For perfect crystals, Eq. (2.6) leads to excellent agreements with the dispersion curves based on atomic lattice theory.

Upon the application of the operator,  $L \equiv 1 - \ell^2\tau^2\nabla^2$  to Eq. (2.2), we obtain

$$(2.8) \quad (1 - \epsilon^2 \tau^2 \nabla^2) t_{k\ell} = \sigma_{k\ell}$$

Divergence of Eq. (2.8), upon using (2.1) and (2.3), leads to

$$(2.9) \quad (\lambda + \mu) u_{k,k\ell} + \mu u_{\ell,kk} + (1 - \epsilon^2 \tau^2 \nabla^2)(\rho f_{\ell} - \rho \ddot{u}_{\ell}) = 0$$

For the static case and vanishing body forces, Eq. (2.9) is non other than Navier's equation of classical elasticity. Note, however, that the stress tensor is not  $\sigma_{k\ell}$  but  $t_{k\ell}$  and it requires that we solve Eq. (2.8) to determine  $t_{k\ell}$ .

For plane, harmonic, SH-waves, Eq. (2.9) gives the frequency

$$(2.10) \quad \omega = (\mu/\rho)^{1/2} k [1 + e_0^2 k^2 a^2]^{-1/2}$$

where  $k$  is the amplitude of the wave vector. By equating  $\omega$  given by Eq. (2.10) to that predicted by the Born-Kármán model of lattice dynamics, at the end of the Brillouin zone ( $ka = \pi$ ), we find that

$$(2.11) \quad e_0 = (\pi^2 - 4)^{1/2} / 2\pi = 0.39.$$

The dispersion curve based on Eq. (2.10) and that of the Born-Kármán model are compared in Fig. 1. We see that the matching is very good. The maximum error is less than 6%.

The dispersion curves of the Born-Kármán model is a good approximation for some fcc and bcc metals (e.g. Al and Cu). While the Brillouin zone may vary in different directions of slips in crystals, we believe that Eq. (2.11) is a reasonable value for  $e_0$  when the material is considered to be isotropic.

### 3. CLASSICAL STRESS FIELDS

A homogeneous, isotropic elastic solid of infinite extent contains a crack located at  $-c \leq x_1 \leq c$ ,  $x_2 = 0$ ,  $-\infty < x_3 < \infty$  where  $x_k$  are the rectangular coordinates, Fig. 2. We suppose that there exists a dislocation which lies parallel to the  $x_3$ -axis and which intersects the plane  $x_3 = 0$  at the point  $S\{x_1 = \xi, x_2 = \eta\}$ . The solid is subject to a constant anti-plane shear at  $x_2 = \pm \infty$ . The classical elasticity solution of this problem was given by Louat<sup>12</sup>. However, here we derive the solution of this problem in the form better suited for our purpose, eliminating possible misprints, difficulties in notations and in taking various limits.

Since the state of the body is the same at all planes,  $x_3 = \text{const.}$ , the problem is two-dimensional and we need to treat the plane problem in the plane  $x_3 = 0$  with a line crack located at  $x_2 = 0$ ,  $|x_1| \leq c$ .

The classical stress field at any point  $P(x_1, x_2)$  may be expressed conveniently in the form

$$(3.1) \quad \sigma_{23} - i \sigma_{13} = A \int_{-c}^c \frac{f(t)}{\bar{z}-t} dt ,$$

where  $\bar{z} = x_1 - i x_2$  and  $f(t)$  is the distribution function which is the solution of the equation of equilibrium of the forces acting on the crack surface:

$$(3.2) \quad A \int_{-c}^{*c} \frac{f(t)}{t-x} dt = \sigma_d(x) + \sigma_0, \quad A = \mu \lambda_0 / 2\pi$$

Here, the integral denotes a Cauchy principal value,  $\mu$  is the shear modulus,  $\lambda_0$  is the displacement vector of a unit positive dislocation and  $\sigma_0$  and  $\sigma_c$  are the stress fields at the crack surface due to the applied load and the dislocation, respectively.

The solution of the integral equation (3.2) is well-known, Tricomi<sup>13</sup>

$$(3.3) \quad f(x) = - \frac{1}{\pi^2 A \sqrt{c^2 - x^2}} \int_{-c}^{*c} \frac{\sqrt{c^2 - t^2}}{\sqrt{c^2 - x^2}} [\sigma_0 + \sigma_d(t)] \frac{dt}{t-x} + \frac{Q}{\sqrt{c^2 - x^2}}$$

Here,  $Q$  is a constant to be determined from the condition that

$$(3.4) \quad \int_{-c}^c f(x) dx = n$$

where  $n\lambda_0$  is the total dislocation content of the distribution  $f(x)$ .

The stress  $\sigma_d(t)$  is given by

$$(3.5) \quad \sigma_d(t) = \frac{\mu b(t-\xi)}{2\pi[(t-\xi)^2 + \eta^2]}$$

Substituting this into Eq. (3.3), we can carry out integrations to obtain

$$(3.6) \quad f(x) = \frac{1}{\sqrt{c^2-x^2}} \left\{ \frac{\sigma_0 x}{\pi A} - \frac{b}{\pi \lambda_0} \left[ \frac{\sqrt{\xi^2-c^2}}{2(\xi-x)} + \frac{\sqrt{\bar{\xi}^2-c^2}}{2(\bar{\xi}-x)} - 1 \right] + Q \right\}$$

where  $\xi = \xi + i\eta$ ,  $\bar{\xi} = \xi - i\eta$ . Using this in Eq. (3.4), we will have  $Q = n/\pi$ . Carrying  $f(x)$  into Eq. (3.1) after some tedious integrations, we obtain

$$(3.7) \quad c_{23} - i \sigma_{13} = \sigma_0 \left( \frac{\bar{z}}{\sqrt{\bar{z}^2-c^2}} - 1 \right) - \frac{bA}{2\lambda_0} \left[ \frac{1}{\bar{z}-\xi} \left( 1 - \frac{\sqrt{\xi^2-c^2}}{\sqrt{\bar{z}^2-c^2}} \right) \right. \\ \left. + \frac{1}{\bar{z}-\bar{\xi}} \left( 1 - \frac{\sqrt{\bar{\xi}^2-c^2}}{\sqrt{\bar{z}^2-c^2}} \right) \right] + \left( \frac{b}{\lambda_0} + n \right) \frac{A}{\sqrt{\bar{z}^2-c^2}}$$

The forces acting on the dislocation at  $(\xi, \eta)$ , due to the crack, are given by

$$(3.8) \quad F_1 = b \sigma_{23}, \quad F_2 = b \sigma_{13} \quad (x_1 = \xi, \quad x_2 = \eta)$$

For our own purpose later, we need the stress field when the dislocation is located along the  $x_1$ -axis and the surface of the crack is free of tractions. To this end, we set  $\eta = 0$  and add

$$(3.9) \quad \sigma_0 + \sigma_d(x_1) = \sigma_0 + \frac{bA}{\lambda_0(\bar{z} - \xi)}$$

to the right-hand side of Eq. (3.7). Hence,

$$(3.10) \quad \sigma_{23} - i \sigma_{13} + \sigma_0 + \sigma_d(x_1) = \frac{1}{\sqrt{\bar{z}^2 - c^2}} \left[ \sigma_0 \bar{z} + A \left( \frac{b}{\lambda_0} + n \right) + \frac{bA}{\lambda_0} \frac{\sqrt{\xi^2 - c^2}}{\bar{z} - \xi} \right]$$

gives the classical stress field at any point outside of the crack when the body is loaded at  $x_2 = \pm \infty$  with a constant shear  $\sigma_{23} = \pm \sigma_0$ . When the crack contains no dislocations, then we have  $n = 0$ .

Two special cases are important:

(i) No Crack and  $\sigma_0 = 0$ . In this case, the classical stress field is given by

$$(3.11) \quad \sigma = \frac{\mu b}{2\pi \bar{z}}$$

where we also set  $\xi = 0$  placing the dislocation to the origin of coordinates.

(ii) No Dislocations. In this case,  $A = 0$  and we have

$$(3.12) \quad \sigma = \frac{\sigma_0 \bar{z}}{\sqrt{\bar{z}^2 - c^2}}$$

Both of these results are well-known in the literature.

#### 4. NONLOCAL STRESS FIELDS

To determine the nonlocal stress fields, we must obtain the solution of

$$(4.1) \quad (1 - \tau_l^2 \nabla^2) t = \sigma$$

subject to some boundary conditions. Here

$$(4.2) \quad t = t_{23} - i t_{13}, \quad \sigma = \sigma_{23} - i \sigma_{13} + \sigma_0 + \sigma_d(x_1)$$

Since  $\nabla^2 \sigma = 0$ ,  $t = \sigma$  is a particular solution of Eq. (4.1). The complementary solution of (4.1), vanishing at infinity and having proper symmetry regulations with respect to  $(x_1, \pm x_2)$ , is of the form

$$(4.3) \quad t_c = K_\nu(r/\lambda_T)(A_\nu e^{i\nu\theta} + B_\nu e^{-i\nu\theta})$$

where  $A_\nu, B_\nu$  and  $\nu$  are constants,  $K_\nu(\rho)$  is the modified Bessel's function and  $(r, \theta)$  are the plane polar coordinates.

The boundary condition on the crack surface requires that  $t_{23} = 0$ . Taking the origin  $r = 0$  of the coordinates at the right-hand crack tip and writing  $r = r_1$ ,  $\theta = \theta_1$ , in (4.3) we see that to fulfill this condition, we must have  $\nu = 1/2$ , since all other solutions lead to displacement singularities at  $r_1 = 0$ .

Classical stress field  $\sigma$  possesses singularity at the screw dislocation  $x_1 = \xi$ ,  $x_2 = 0$ . The surface traction,  $t_{r2}$  on the edge surface of the dislocation is required to vanish, according to the boundary



condition. To fulfill this condition we take  $\nu=1$  and move the origin of coordinates to  $x_1=\xi$ ,  $x_2=0$ . This may be expressed by writing  $r=r_d$ ,  $\theta=\theta_3$  and

$$(4.4) \quad r_d e^{i\theta_3} = r_1 e^{i\theta_1} - x_0.$$

Hence, the general solution of (4.1) appropriate to our problem is of the form

$$(4.5) \quad t = (\pi\tau\ell/2r_1)^{\frac{1}{2}} e^{-r_1/\tau\ell} (C_1 e^{i\theta_1/2} + C_2 e^{-i\theta_1/2}) \\ + K_1(r_d/\tau\ell)(C_3 e^{i\theta_3} + C_4 e^{-i\theta_3}) + \sigma.$$

To determine  $C_\alpha$ , we calculate stress components in polar coordinates  $(r_1, \theta_1)$ :

$$(4.6) \quad t_{\theta z} - i t_{rz} = (t_{23} - i t_{13}) e^{-i\theta_1}.$$

We imagine the crack tip as a limit of a small circular arc with radius  $r_1=\epsilon$  approaching zero. For small  $\epsilon$ , we have approximately

$$(4.7) \quad z = c + z_1 = -c + z_2 = \xi + z_3 = c + x_0 + z_3, \\ z_1 = \epsilon e^{i\theta_1}.$$

Using these in Eq. (3.10), we will have

$$(4.8) \quad \sigma = (c/2r_1)^{\frac{1}{2}} e^{i\theta_1/2} \left[ \sigma_0 + \frac{\mu b}{2\pi c} \left( 1 + \frac{n\lambda_0}{b} \right) - \frac{\mu b}{2\pi c} \left( 1 + \frac{2c}{x_0} \right)^{\frac{1}{2}} \right]$$

Consequently, Eq. (4.5) gives

$$(4.9) \quad t_{\theta z} - i t_{rz} = (c/2r_1)^{\frac{1}{2}} e^{-i\theta_1/2} \left[ \sigma_0 + \frac{\mu b}{2\pi c} \left( 1 + \frac{n\lambda_0}{b} \right) - \frac{\mu b}{2\pi c} \left( 1 + \frac{2c}{x_0} \right)^{\frac{1}{2}} \right] \\ + (\pi\tau\ell/2r_1)^{\frac{1}{2}} e^{-r_1/\tau\ell} (C_1 e^{-i\theta_1/2} + C_2 e^{-3i\theta_1/2}) \\ + K_1(|x_0|/\tau\ell) (C_3 \frac{r_1}{x_0} e^{i\theta_1} - C_3 + C_4 \frac{r_1}{x_0} e^{-i\theta_1} - C_4) e^{-i\theta_1}$$

The boundary condition on  $t_{rz}$  requires that

$$(4.10) \quad \lim_{r_1 \rightarrow 0} t_{rz} = 0$$

This condition will be fulfilled approximately\* for  $x_0/\tau\ell \gg 1$  if  $C_2 = 0$  and

$$(4.11) \quad C_1 = - (c/\pi\tau\ell)^{\frac{1}{2}} \left[ \sigma_0 + \frac{\mu b}{2\pi c} \left( 1 + \frac{n\lambda_0}{b} \right) - \frac{\mu b}{2\pi c} \left( 1 + \frac{2c}{x_0} \right)^{\frac{1}{2}} \right]$$

Next, we calculate the stress field at the location  $r_d = 0$  of the screw dislocation. As  $r_d \rightarrow 0$  we have  $r_1 = x_0$  and

---

\*In fact, this condition is satisfied exactly along the crack line  $\theta_1 = \pm\pi$ .

$$\begin{aligned}
 (4.12) \quad t_{z\theta} - i t_{zr} = & (\pi\tau\ell/2x_0)^{\frac{1}{2}} e^{-x_0/\tau\ell} C_1 + K_1(r_d/\tau\ell)(C_3 e^{i\theta_3} + C_4 e^{-i\theta_3}) \\
 & + [x_0(x_0 + 2c)]^{-\frac{1}{2}} [\sigma_0(c + x_0) + \frac{\mu b}{2\pi} (1 + \frac{n\lambda_0}{b}) \\
 & + \frac{\mu b}{2\pi r_d} [x_0(x_0 + 2c)]^{\frac{1}{2}} e^{i\theta_3} .
 \end{aligned}$$

Again  $t_{zr}$  must vanish as  $r_d \rightarrow 0$ . This implies that  $C_4 = 0$  and

$$(4.13) \quad C_3 = -\mu b/2\pi\tau\ell .$$

The general solution is now complete.

$$(4.14) \quad t = (\pi\tau\ell/2r_1)^{\frac{1}{2}} e^{-r_1/\tau\ell} C_1 e^{i\theta_1/2} + K_1(r_d/\tau\ell) C_3 e^{i\theta_3} + \sigma$$

where  $C_1$  and  $C_3$  are given by (4.11) and (4.13). In polar coordinates, we have

$$\begin{aligned}
 (4.15) \quad t_{\theta z} - i t_{rz} = & (\pi\tau\ell/2r_1)^{\frac{1}{2}} e^{-r_1/\tau\ell} C_1 e^{-i\theta_1/2} + K_1(r_d/\tau\ell) C_3 e^{i(\theta_3 - \theta_1)} \\
 & + (r_1 r_2)^{-\frac{1}{2}} e^{i(\theta_2 - \theta_1)/2} \left\{ [\sigma_0 r e^{-i\theta} + \frac{\mu b}{2\pi} (1 + \frac{n\lambda_0}{b}) \right. \\
 & \left. + \frac{\mu b}{2\pi} (r_1 e^{-i\theta_1} - x_0)^{-1} [x_0(x_0 + 2c)]^{\frac{1}{2}} \right\}
 \end{aligned}$$

Special cases mentioned in Section 3 can be obtained in a similar fashion.

(i) No Crack

$$(4.16) \quad t_{\theta z} - i t_{rz} = \frac{\mu b}{2\pi\tau\ell} \frac{1}{\rho} [1 - \rho K_1(\rho)] ,$$

where we have taken the origin of the polar coordinates at the dislocation, so that  $\rho = r_d/\tau\ell$ .

(ii) No Dislocation

$$(4.17) \quad t_{\theta z} - i t_{rz} = \sigma_0 (c/2r_1)^{\frac{1}{2}} [(2r^2/cr_2)^{\frac{1}{2}} e^{i(-\theta + \frac{2}{2})} - e^{-r_1/\tau\ell}] e^{-i\theta_1/2}$$

## 5. FRACTURE

Here, we discuss the onset of fracture and determine the theoretical stresses for the two special cases.

(i) No Crack

According to Eq. (4.16), we have  $t_{rz} \approx 0$  and

$$(5.1) \quad T_{\theta}(\rho) = \frac{2\pi\tau\ell}{\mu b} t_{\theta z} = \rho^{-1} [1 - \rho K_1(\rho)]$$

The maximum of  $T_{\theta}$  occurs at  $\rho = 1.1$  and is given by

$$(5.2) \quad T_{\theta\max} = 0.3993; \quad \rho_c = 1.1$$

It is natural to assume that when  $t_{\theta z \max}$  becomes equal to the theoretical stress  $t_y$ , the crystal will rupture. Thus,

$$(5.3) \quad t_y/\mu \approx 0.3993 \frac{b}{2\pi e_0 a}$$

If we write  $h = e_0 a / 0.3993$ , this agrees with the estimate of Frenkel based on an atomic model (cf. Kelly<sup>14</sup>, p. 12). For aluminum (fcc),  $b = a/\sqrt{2}$  and for iron (bcc),  $b = \sqrt{3}/2$ , so that Eq. (5.3) gives

$$t_y/\mu = 0.12 \quad \{\text{Al: } [111] \langle 1\bar{1}0 \rangle\}$$

$$t_y/\mu = 0.14 \quad \{\text{Fe: } [1\bar{1}0] \langle 111 \rangle\}$$

These are close to the theoretical results  $t_y/\mu = 0.11$  based on atomic models.

It is interesting to note that  $t_{\theta z} = 0$  at the center of dislocation and it rises to a maximum at  $\rho = 1.1$ , thereafter decreasing to zero with  $\rho$ . Significant consequences of the present predictions as contrasted to the classical results are:

- (a) The stress at the center of the core is not infinite, but zero.
- (b) Fracture begins at  $\rho = \rho_c$  not at the center of the core.
- (c) There is a low stress region,  $0 < \rho < \rho_c$  within the core.

(ii) No Dislocations

From (4.17), it is clear that  $t_{\theta z}$  acquires its maximum along the crack line  $\theta = \theta_1 = \theta_2 = 0$ , near the crack tip. The circumferential stress along

the crack line  $r_1 \geq 0$  is expressed by

$$(5.4) \quad t_{z\theta}/\sigma_0 = (2\gamma\rho)^{-\frac{1}{2}} \left[ (1 + \gamma\rho) \left(1 + \frac{\gamma\rho}{2}\right)^{-\frac{1}{2}} - e^{-\rho} \right]$$

where

$$(5.5) \quad \rho = r_1/e_0a, \quad \gamma = e_0a/c$$

$t_{z\theta}$  vanishes at the crack tip  $\rho=0$  and has a maximum at  $\rho=\rho_c$  which is the root of

$$(5.6) \quad e^{-\rho} (1 + 2\rho) = \left(1 + \frac{\gamma\rho}{2}\right)^{-3/2}$$

Since  $\gamma \ll 1$  ( $\gamma \leq 10^{-6}$ ), we see that the root of (5.6) is independent of  $c$  and is given by

$$(5.7) \quad \rho_c = 1.2565$$

and the maximum stress is given by

$$(5.8) \quad t_{z\theta \max} = \sigma_0 (e_0a/c)^{-\frac{1}{2}} \left( \sqrt{2\rho_c} + \frac{1}{\sqrt{2\rho_c}} \right)^{-1}.$$

We also observe that as  $\rho \rightarrow \infty$ , (5.4) gives  $t_{z\theta} = \sigma_0$ , as it should.

In the classical tradition, if we write  $K_{III} = \sqrt{\pi c} \sigma_0$ , then (5.4) may be expressed as

$$\begin{aligned}
 (5.9) \quad T_{\theta}(\rho) &= \sqrt{\gamma} t_{z\theta}/\sigma_0 = (\pi e_0 a)^{\frac{1}{2}} t_{z\theta}/K_{III} \\
 &= (2\rho)^{-\frac{1}{2}} [1 + \gamma\rho] (1 + \frac{\gamma\rho}{2})^{-\frac{1}{2}} - e^{-\rho}
 \end{aligned}$$

This is plotted against  $\rho$ , in Fig. 3 in the vicinity of the crack tip. The classical (local) stress is also indicated on this figure by dashed lines. From this figure, it is clear that the classical stress field deviates considerably from the nonlocal stress field in the region  $0 \leq \rho < 5$ . In fact, it diverges at the crack tip.

A perfect crystal which contains a crack, but no dislocation, will not rupture before the maximum stress reaches the value of the cohesive stress (theoretical stress) that holds the atomic bonds of the lattice. Thus, the entire crystal is in the elastic state of equilibrium when

$$(5.10) \quad t_{z\theta \max} < t_y.$$

The failure begins when  $t_{z\theta \max} = t_y$ , i.e., when

$$(5.11) \quad K_c/t_y = (\pi e_0 a)^{\frac{1}{2}} (\sqrt{2\rho_c} + \frac{1}{\sqrt{2\rho_c}}) = 3.9278 \sqrt{e_0 a}$$

where  $K_c = \sqrt{\pi c} \sigma_{0c}$  is the critical fracture toughness.

Using Eq. (5.11) and  $e_0 = 0.39$ , we calculate a few  $K_g$ -values which are listed in Table 1 (last column) along with classical  $K_c$ 's based on  $K_c = 4\mu\gamma_s^{\frac{1}{2}}$ , where  $\gamma_s$  is the surface energy. Experimental observations

of Ohr and Chang<sup>15,16</sup> are also listed in this Table. Classical estimates are expected to be inaccurate considering the fact that even with the best present-day techniques available, they could not be measured to an accuracy better than a factor of two. Moreover, classical formula assumes no defects (i.e., no crack and dislocation), therefore it is expected to give higher  $K_c$ -values. On the other hand, experimental measurements of Ohr and Chang required the measurement of the length of the plastic zone among other constants. This already implies the existence of dislocations so that we expect some deviation from the perfect crystal containing no dislocation but a single crack. In Section 6, we examine the general case when the solid contains a crack and a dislocation.

## 6. DISLOCATION AND CRACK

Along the crack line  $x_1 \geq c$ ,  $x_2 = 0$ ,  
Eq. (4.15) gives  $t_{rz} = 0$  and with  $n = 0$

$$(6.1) \quad t_{\theta z} = t^c + t^{dc}$$

where

$$(6.2) \quad t^c \sqrt{\gamma} / \sigma_0 \equiv T_1 = (2c)^{-1/2} [1 + \gamma \rho] (1 + \frac{\gamma c}{2})^{-1/2} - e^{-c},$$

$$(6.3) \quad t^{dc} \sqrt{\gamma} / \sigma_0 \equiv T_2 = (2c)^{-1/2} \left\{ \left[ (1 + \frac{\gamma c}{2})^{-1/2} - \left( 1 - (1 + \frac{2}{\gamma \bar{x}_0})^{1/2} \right) e^{-c} \right] \right. \\ \left. - \operatorname{sgn}(\rho - \bar{x}_0) (2\rho/\gamma)^{1/2} K_1(|\rho - \bar{x}_0|) + (1 + \frac{\gamma c}{2})^{-1/2} (1 + \frac{2}{\gamma \bar{x}_0})^{1/2} (\frac{c}{\bar{x}_0} - 1)^{-1} \right\}$$



in which

$$(6.4) \quad \bar{x}_0 = x_0/e_0 a, \quad \beta = \frac{\mu b}{2\pi c \sigma_0}$$

when the dislocation is absent we have  $t^{dc} = 0$  so that  $t^{dc}$  is the shear stress arising from the interaction of the dislocation with the crack. At the crack tip  $\rho = 0$  and we have

$$(6.5) \quad t_{\theta z} / \sigma_0 \beta = \frac{1}{\gamma} K_1(\bar{x}_0)$$

This shows that for positive dislocation ( $b > 0$ ) the shear stress is positive and therefore the crack tip will tend to close up for  $b > 0$ . For  $b < 0$  the opposite will occur. However, the stress given by (6.5) due to the dislocation is very small for large  $\bar{x}_0$  and it becomes large when the dislocation is very close to the crack tip. The stress field  $T_1(\cdot) \equiv T_\epsilon(\cdot)$  so that Fig. 3 represents  $T_1(\rho)$ . Fig. 4 displays graphs of  $T_2(\cdot)$  for several values of  $\bar{x}_0 = 3.1, 4.1, 6.1$  and  $10.1$ , keeping  $\beta = 10^{-8}$  fixed. These graphs show that  $T_2$  possesses a minimum and two maxima. The crack tip is not stress-free. The maximum of  $T_2$  occur close to the dislocation. For example, in the case  $\bar{x}_0 = 3.1$ , the maximum is at  $\bar{x}_c = 4.4$  and in the case  $\bar{x}_0 = 10.1$  it is at  $\bar{x}_c = 11.2$ .

To obtain an idea on the combined effect I have selected  $\beta = 10^{-4}$  and plotted the ratio of the combined stress to  $\sigma_0(t_{\theta z}/\sigma_0)$  in Fig. 5 for various  $\bar{x}_0$ . From these graphs it is clear that when

the dislocation is located close to the maximum of  $T_1$  the combined effect is large. For example, while  $T_{1\max} \approx 0.451$  for the case of crack alone, for the combined effect we have  $T_{\max} = 0.63$  so that the ratio of the two  $K_g$ -values is given by

$$(6.6) \quad K_{g\text{tot}} / K_{g1} \approx 0.71$$

This implies that a dislocation located at a distance approximately one or two lattice parameters away from the crack tip reduces the fracture toughness by about 30%. Hence the theoretical values of  $K_g$  listed in Table 1 will be reduced about 30% bringing the numbers on the last column closer to those listed in the adjacent column marked experiments. These results however must still be considered only as indications for the trend. A more realistic physical picture requires the presence of large numbers of dislocations distributed over a few microns or so distance, away from the crack tip. Consequently, to obtain a close approximation to experimental observations of Chang and Ohr we need to consider a distribution of dislocation in a region near the crack tip. Such a consideration will require a separate study of dislocation pile up which is left to a future study.

#### Acknowledgment

Present work was supported by the Office of Naval Research. The author is indebted to Dr. N. Basdekas for his encouragement and enthusiasm. I wish to thank Drs. S.M. Ohr and S.-J. Chang, for many valuable discussions and Mr. A. Suresh for the computer work.

# REFERENCES

- [1] J.P. Hirth and J. Lothe, "Theory of Dislocations", McGraw-Hill, New York, 1968, Ch. 8.
- [2] A.A. Griffith, Phil. Trans. Roy. Soc., A221, 163, 1920.
- [3] A.C. Eringen, C.G. Speziale and B.S. Kim, J. Mech. Phys. Solids, 25, 339 (1977).
- [4] A.C. Eringen, Int. J. of Fracture, 14, 367 (1978)
- [5] A.C. Eringen, Nonlinear Equations in Physics and Mathematics (Edited by A.O. Barut), Reidel Pub. Co., Holland, 271-318 (1978).
- [6] A.C. Eringen, J. Phys. D. Appl. Phys., 10, 671 (1977).
- [7] A.C. Eringen, and D.G.B. Edelen, Int. J. Engng. Sci., 10, 233 (1972).
- [8] A.C. Eringen, Int. J. Engng. Sci., 10, 425 (1972).
- [9] A.C. Eringen, Continuum Physics, Vol. IV, 204-267, Academic Press (1976).
- [10] N. Ari, and A.C. Eringen, "Nonlocal Stress Field at Griffith Crack," Crystal Lattice Defects and Amorph. Mat. 10, 33, 1983.
- [11] A.C. Eringen and B.S. Kim, Crystal Lattice Defects, 7, 51 (1977).
- [12] N.P. Louat, Proc. Int. Conf. on Fracture, 117, Sendai, Japan (1965).
- [13] F.G. Tricomi, Quart. J. Math. Oxford 2, 199, 1951.
- [14] A. Kelly, Strong Solids, 12, Oxford 1966.
- [15] S.M. Ohr, J.A. Horton and S.-J. Chang, "Direct Observations of Crack Tip Dislocation Behavior During Tensile and Cyclic Deformation," Technical Report, Oak Ridge National Laboratory.
- [16] S.M. Ohr, and S.-J. Chang, J. Appl. Phys., 53, 5645, 1982.
- [17] C. Kittel, Introduction to Solid State Physics, 4th ed. p.38, John Wiley, 1974.

- [18] J. Rice and R. Thomson, Phil. Mag., 29, 73 (1974).
- [19] A. Kelly, W.R. Tyson and A.H. Cottrell, Phil. Mag. 15, 567 (1967)

Table 1 : Critical Stress Intensity Factors

Material	$a(10^{-8} \text{ cm})$ <sup>17</sup>	$\mu(10^{11} \text{ cgs})$ <sup>18</sup>	$\gamma_s(\text{cgs})$ <sup>18</sup>	$t_y(10^{11} \text{ cgs})$ <sup>19</sup>	Classical $K_c/t_y(10^{-3} \text{ cm}^{\frac{1}{2}})$	Experiment <sup>16</sup> $K/t_y(10^{-3} \text{ cm}^{\frac{1}{2}})$	Present $K_R/t_y(10^{-3} \text{ cm}^{\frac{1}{2}})$
Al (fcc)	4.05	2.51	840	0.262	1.11	0.31	0.49
Cu (fcc)	3.61	4.05	1688	0.137	3.86	0.66	0.47
Ni (fcc)	3.52	7.48	1725	0.274	2.62	0.66	0.46
Fe (bcc)	2.87	6.9	1975	0.71	1.04	0.23	0.42

## LIST OF FIGURES

- Figure 1: Dispersion Curves for the Present Nonlocal Model and the Born-Kármán Model of Lattice Dynamics
- Figure 2: Crack Subject to Anti-Plane Shear (Mode III)
- Figure 3: Non-Dimensional Shear (No Dislocation)
- Figure 4: Non-Dimensional Shear Stress Due to Dislocation Interaction
- Figure 5: Total Shear Stress (Crack and Dislocation)

## LIST OF TABLES

- Table 1: Critical Stress Intensity Factors

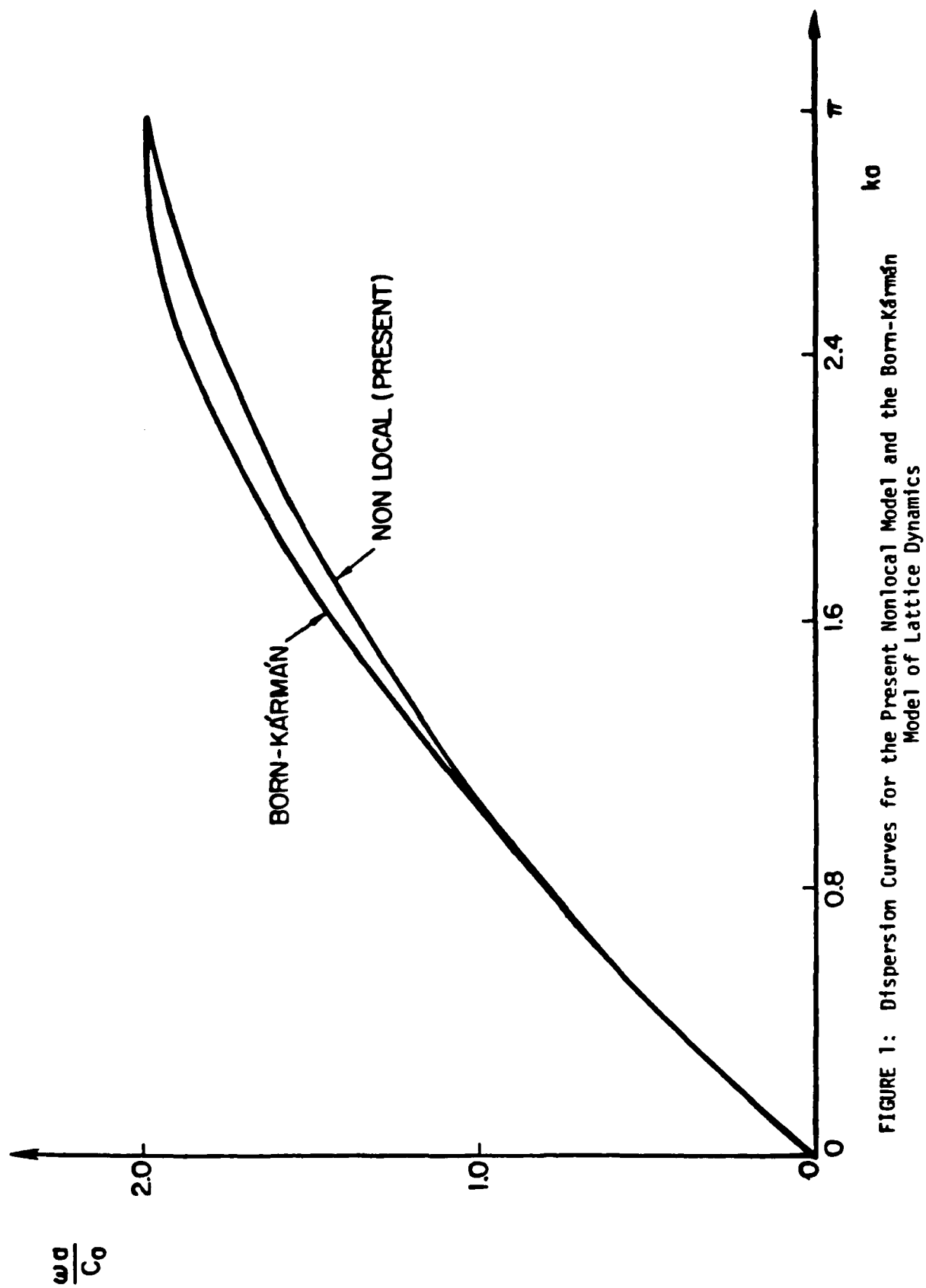
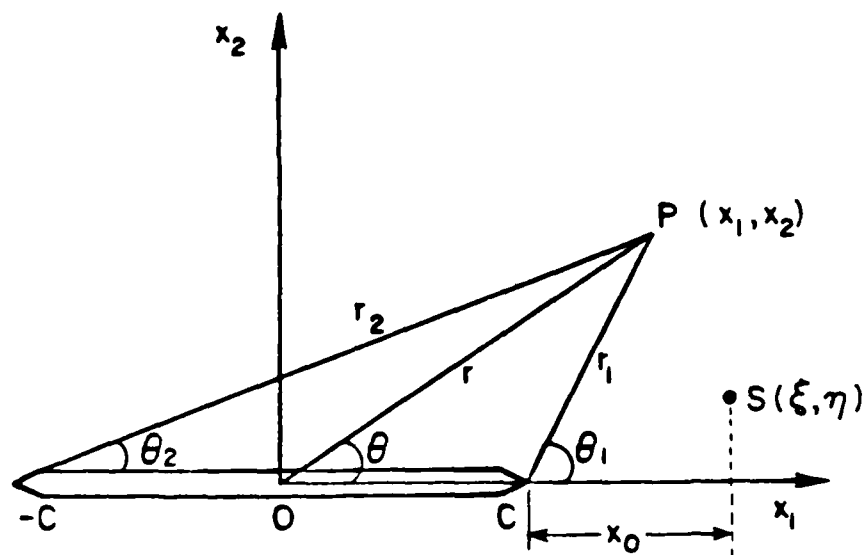


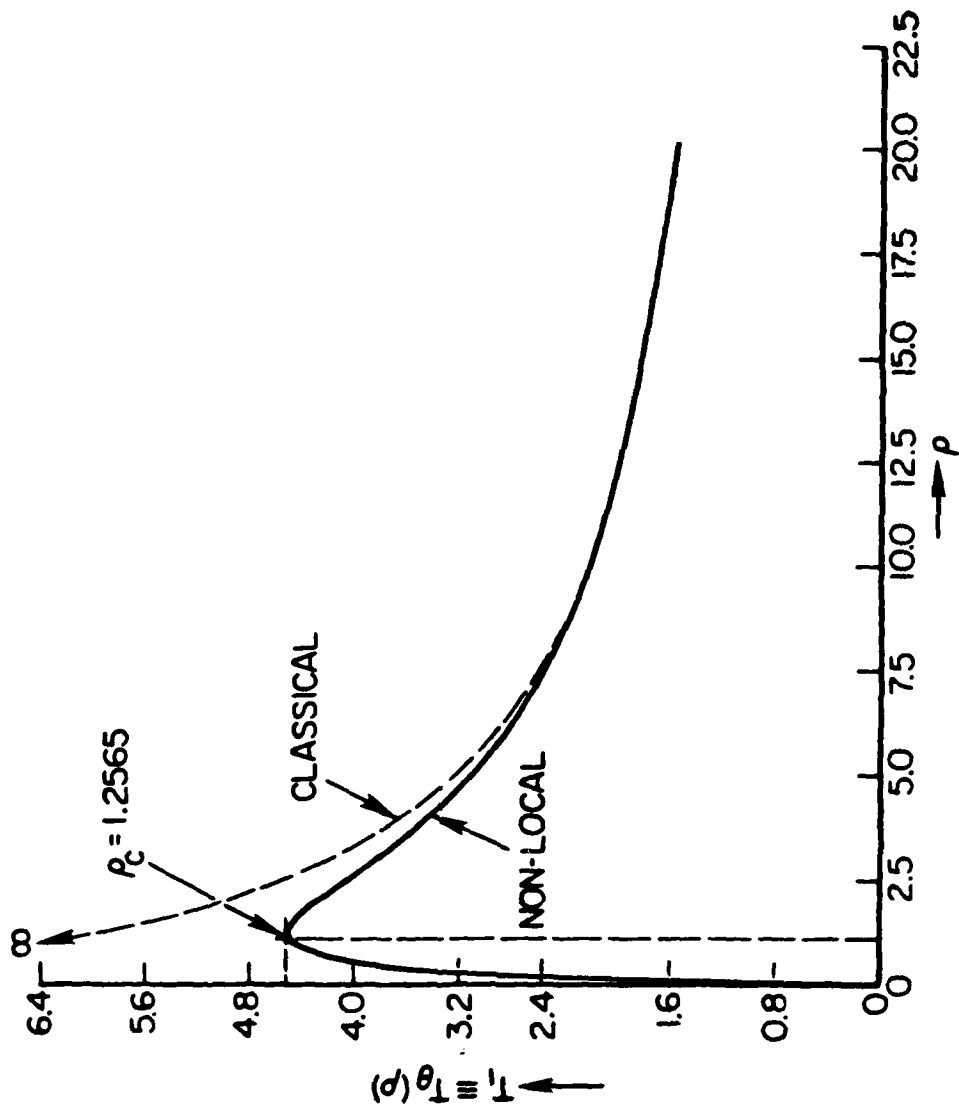
FIGURE 1: Dispersion Curves for the Present Nonlocal Model and the Born-Kármán Model of Lattice Dynamics



CRACK SUBJECT TO ANTI-PLANE SHEAR (MODE III)

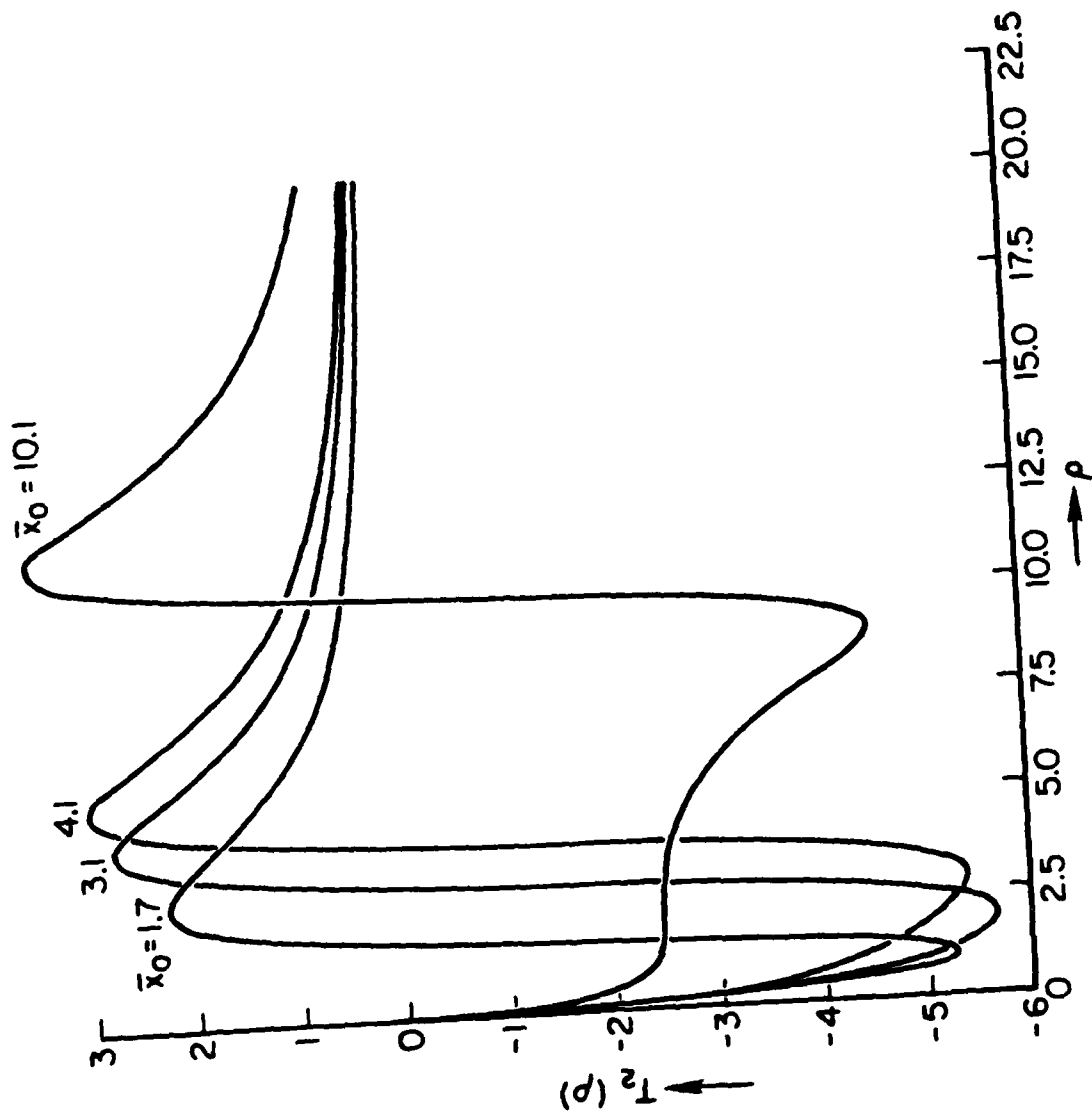
FIGURE 2





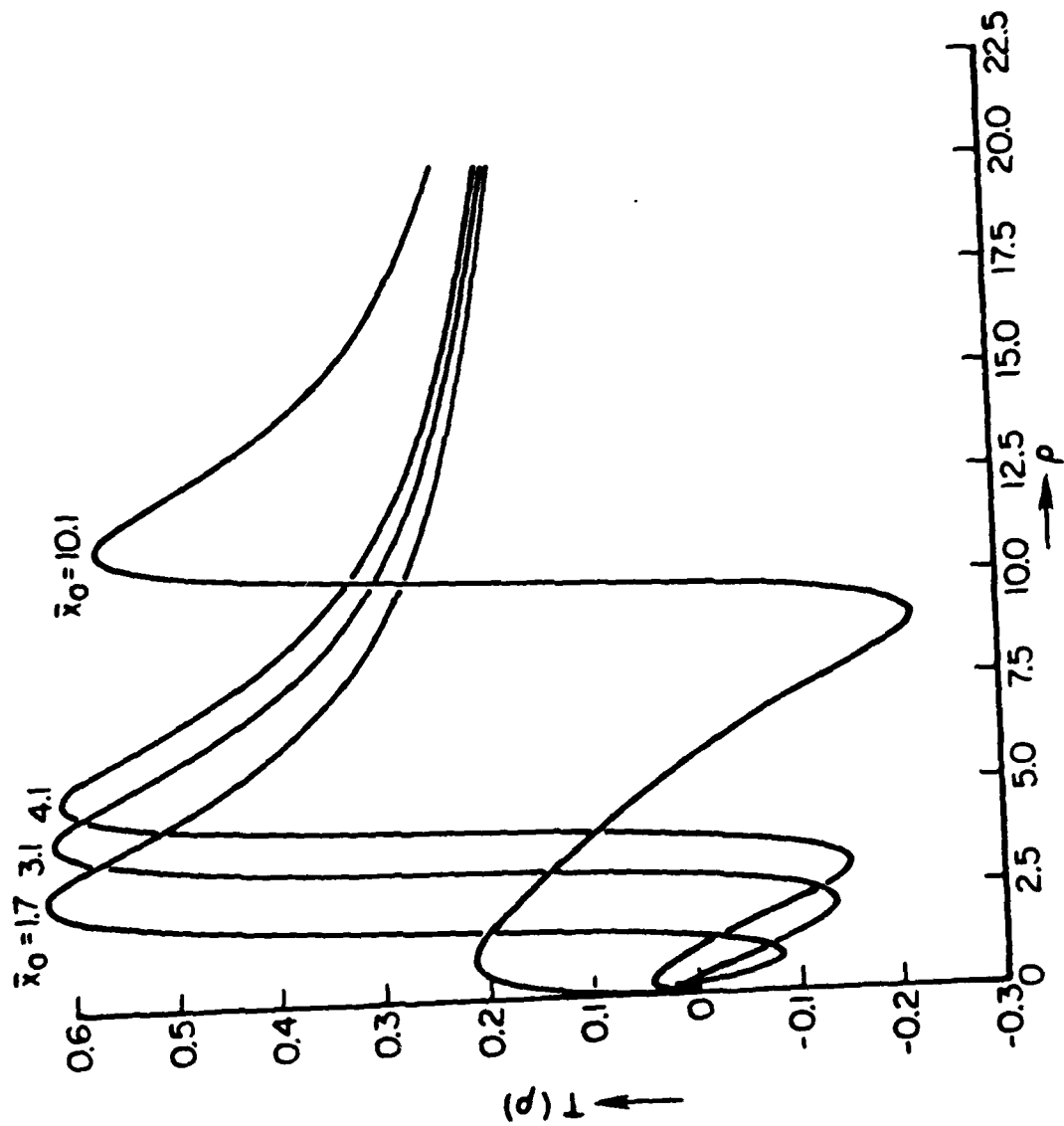
NON-DIMENSIONAL SHEAR (NO DISLOCATION)

FIGURE 3



NON-DIMENSIONAL SHEAR STRESS DUE TO DISLOCATION INTERACTION

FIGURE 4



TOTAL SHEAR STRESS (CRACK AND DISLOCATION)

FIGURE 5

**Part 1 - Government**  
**Administrative and Liaison Activities**

Office of Naval Research  
Department of the Navy  
Arlington, Virginia 22217  
Attn: Code 474 (2)  
Code 471  
Code 200

Director  
Office of Naval Research  
Eastern/Central Regional Office  
666 Summer Street  
Boston, Massachusetts 02210

Director  
Office of Naval Research  
Branch Office  
536 South Clark Street  
Chicago, Illinois 60605

Director  
Office of Naval Research  
New York Area Office  
715 Broadway - 5th Floor  
New York, New York 10003

Director  
Office of Naval Research  
Western Regional Office  
1030 East Green Street  
Pasadena, California 91106

Naval Research Laboratory (6)  
Code 2627  
Washington, D.C. 20375

Defense Technical Information Center (12)  
Cameron Station  
Alexandria, Virginia 22314

**Navv**

Undersea Explosion Research Division  
Naval Ship Research and Development  
Center  
Norfolk Naval Shipyard  
Portsmouth, Virginia 23709  
Attn: Dr. E. Palmer, Code 177

**Navy (Con't.)**

Naval Research Laboratory  
Washington, D.C. 20375  
Attn: Code 8400  
8410  
8430  
8440  
6300  
6390  
6380

David W. Taylor Naval Ship Research  
and Development Center  
Annapolis, Maryland 21402  
Attn: Code 2740  
28  
281

Naval Weapons Center  
China Lake, California 93555  
Attn: Code 4062  
4520

Commanding Officer  
Naval Civil Engineering Laboratory  
Code L31  
Port Hueneme, California 93041

Naval Surface Weapons Center  
White Oak  
Silver Spring, Maryland 20910  
Attn: Code R-10  
G-402  
K-82

Technical Director  
Naval Ocean Systems Center  
San Diego, California 92152

Supervisor of Shipbuilding  
U.S. Navy  
Newport News, Virginia 23607

Navy Underwater Sound  
Reference Division  
Naval Research Laboratory  
P.O. Box 8337  
Orlando, Florida 32806

Chief of Naval Operations  
Department of the Navy  
Washington, D.C. 20350  
Attn: Code OP-098

474:NP:716:lab  
78u474-619

Navy (Con't.)

Strategic Systems Project Office  
Department of the Navy  
Washington, D.C. 20376  
Attn: MSP-200

Naval Air Systems Command  
Department of the Navy  
Washington, D.C. 20361  
Attn: Code 5302 (Aerospace and Structures)  
604 (Technical Library)  
320B (Structures)

Naval Air Development Center  
Warminster, Pennsylvania 18974  
Attn: Aerospace Mechanics  
Code 606

U.S. Naval Academy  
Engineering Department  
Annapolis, Maryland 21402

Naval Facilities Engineering Command  
200 Stovall Street  
Alexandria, Virginia 22332  
Attn: Code 03 (Research and Development)  
04B  
045  
14114 (Technical Library)

Naval Sea Systems Command  
Department of the Navy  
Washington, D.C. 20362  
Attn: Code 05H  
312  
322  
323  
05R  
32R

Navy (Con't.)

Commander and Director  
David W. Taylor Naval Ship  
Research and Development Center  
Bethesda, Maryland 20084  
Attn: Code 042

17  
172  
173  
174  
1800  
1844  
012.2  
1900  
1901  
1945  
1960  
1962

Naval Underwater Systems Center  
Newport, Rhode Island 02840  
Attn: Bruce Sandman, Code 3634

Naval Surface Weapons Center  
Dahlgren Laboratory  
Dahlgren, Virginia 22448  
Attn: Code G04  
G20

Technical Director  
Mare Island Naval Shipyard  
Vallejo, California 94592

U.S. Naval Postgraduate School  
Library  
Code 0384  
Monterey, California 93940

Webb Institute of Naval Architecture  
Attn: Librarian  
Crescent Beach Road, Glen Cove  
Long Island, New York 11542

Army

Commanding Officer (2)  
U.S. Army Research Office  
P.O. Box 12211  
Research Triangle Park, NC 27709  
Attn: Mr. J. J. Murray, CRD-AA-IP

Army (Con't.)

Watervliet Arsenal  
MAGGS Research Center  
Watervliet, New York 12189  
Attn: Director of Research

U.S. Army Materials and Mechanics  
Research Center  
Watertown, Massachusetts 02172  
Attn: Dr. R. Shea, DRXMR-T

U.S. Army Missile Research and  
Development Center  
Redstone Scientific Information  
Center  
Chief, Document Section  
Redstone Arsenal, Alabama 35809

Army Research and Development  
Center  
Fort Belvoir, Virginia 22060

NASA

National Aeronautics and Space  
Administration  
Structures Research Division  
Langley Research Center  
Langley Station  
Hampton, Virginia 23365

National Aeronautics and Space  
Administration  
Associate Administrator for Advanced  
Research and Technology  
Washington, D.C. 20546

Air Force

Wright-Patterson Air Force Base  
Dayton, Ohio 45433  
Attn: AFFDL (FB)  
          (FBR)  
          (FBE)  
          (FBS)  
      AFML (MBM)

Chief Applied Mechanics Group  
U.S. Air Force Institute of Technology  
Wright-Patterson Air Force Base  
Dayton, Ohio 45433

Air Force (Con't.)

Chief, Civil Engineering Branch  
WLRC, Research Division  
Air Force Weapons Laboratory  
Kirtland Air Force Base  
Albuquerque, New Mexico 87117

Air Force Office of Scientific Research  
Bolling Air Force Base  
Washington, D.C. 20332  
Attn: Mechanics Division

Department of the Air Force  
Air University Library  
Maxwell Air Force Base  
Montgomery, Alabama 36112

Other Government Activities

Commandant  
Chief, Testing and Development Division  
U.S. Coast Guard  
1300 E Street, NW.  
Washington, D.C. 20226

Technical Director  
Marine Corps Development  
and Education Command  
Quantico, Virginia 22134

Director Defense Research  
and Engineering  
Technical Library  
Room 3C128  
The Pentagon  
Washington, D.C. 20301

Dr. M. Gaus  
National Science Foundation  
Environmental Research Division  
Washington, D.C. 20550

Library of Congress  
Science and Technology Division  
Washington, D.C. 20540

Director  
Defense Nuclear Agency  
Washington, D.C. 20305  
Attn: SPSS

Other Government Activities (Con't)

Mr. Jerome Persh  
Staff Specialist for Materials  
and Structures  
OUSDR&E, The Pentagon  
Room 3D1089  
Washington, D.C. 20301

Chief, Airframe and Equipment Branch  
FS-120  
Office of Flight Standards  
Federal Aviation Agency  
Washington, D.C. 20553

National Academy of Sciences  
National Research Council  
Ship Hull Research Committee  
2101 Constitution Avenue  
Washington, D.C. 20418  
Attn: Mr. A. R. Lytle

National Science Foundation  
Engineering Mechanics Section  
Division of Engineering  
Washington, D.C. 20550

Picatinny Arsenal  
Plastics Technical Evaluation Center  
Attn: Technical Information Section  
Dover, New Jersey 07801

Maritime Administration  
Office of Maritime Technology  
14th and Constitution Avenue, NW.  
Washington, D.C. 20230

**PART 2 - Contractors and Other Technical  
Collaborators**

Universities

Dr. J. Tinsley Oden  
University of Texas at Austin  
345 Engineering Science Building  
Austin, Texas 78712

Professor Julius Miklowitz  
California Institute of Technology  
Division of Engineering  
and Applied Sciences  
Pasadena, California 91109

Universities (Con't)

Dr. Harold Liebowitz, Dean  
School of Engineering and  
Applied Science  
George Washington University  
Washington, D.C. 20052

Professor Eli Sternberg  
California Institute of Technology  
Division of Engineering and  
Applied Sciences  
Pasadena, California 91109

Professor Paul M. Naghdi  
University of California  
Department of Mechanical Engineering  
Berkeley, California 94720

Professor A. J. Durelli  
Oakland University  
School of Engineering  
Rochester, Missouri 48063

Professor F. L. DiMaggio  
Columbia University  
Department of Civil Engineering  
New York, New York 10027

Professor Norman Jones  
The University of Liverpool  
Department of Mechanical Engineering  
P. O. Box 147  
Brownlow Hill  
Liverpool L69 3BX  
England

Professor E. J. Skudrzyk  
Pennsylvania State University  
Applied Research Laboratory  
Department of Physics  
State College, Pennsylvania 16801

Professor J. Klosner  
Polytechnic Institute of New York  
Department of Mechanical and  
Aerospace Engineering  
333 Jay Street  
Brooklyn, New York 11201

Professor R. A. Schapery  
Texas A&M University  
Department of Civil Engineering  
College Station, Texas 77843

Universities (Con't.)

Professor Walter D. Pilkey  
University of Virginia  
Research Laboratories for the  
Engineering Sciences and  
Applied Sciences  
Charlottesville, Virginia 22901

Professor K. D. Willmert  
Clarkson College of Technology  
Department of Mechanical Engineering  
Potsdam, New York 13676

Dr. Walter E. Haisler  
Texas A&M University  
Aerospace Engineering Department  
College Station, Texas 77843

Dr. Hussein A. Kamel  
University of Arizona  
Department of Aerospace and  
Mechanical Engineering  
Tucson, Arizona 85721

Dr. S. J. Fenves  
Carnegie-Mellon University  
Department of Civil Engineering  
Schenley Park  
Pittsburgh, Pennsylvania 15213

Dr. Ronald L. Huston  
Department of Engineering Analysis  
University of Cincinnati  
Cincinnati, Ohio 45221

Professor G. C. M. Sih  
Lehigh University  
Institute of Fracture and  
Solid Mechanics  
Bethlehem, Pennsylvania 18015

Professor Albert S. Kobayashi  
University of Washington  
Department of Mechanical Engineering  
Seattle, Washington 98105

Professor Daniel Frederick  
Virginia Polytechnic Institute and  
State University  
Department of Engineering Mechanics  
Blacksburg, Virginia 24061

Universities (Con't)

Professor A. C. Eringen  
Princeton University  
Department of Aerospace and  
Mechanical Sciences  
Princeton, New Jersey 08540

Professor E. H. Lee  
Stanford University  
Division of Engineering Mechanics  
Stanford, California 94305

Professor Albert I. King  
Wayne State University  
Biomechanics Research Center  
Detroit, Michigan 48202

Dr. V. R. Hodgson  
Wayne State University  
School of Medicine  
Detroit, Michigan 48202

Dean B. A. Boley  
Northwestern University  
Department of Civil Engineering  
Evanston, Illinois 60201

Professor P. G. Hodge, Jr.  
University of Minnesota  
Department of Aerospace Engineering  
and Mechanics  
Minneapolis, Minnesota 55455

Dr. D. C. Drucker  
University of Illinois  
Dean of Engineering  
Urbana, Illinois 61801

Professor N. M. Newmark  
University of Illinois  
Department of Civil Engineering  
Urbana, Illinois 61803

Professor E. Reissner  
University of California, San Diego  
Department of Applied Mechanics  
La Jolla, California 92037

Professor William A. Nash  
University of Massachusetts  
Department of Mechanics and  
Aerospace Engineering  
Amherst, Massachusetts 01002



Universities (Con't)

Professor G. Herrmann  
Stanford University  
Department of Applied Mechanics  
Stanford, California 94305

Professor J. D. Achenbach  
Northwest University  
Department of Civil Engineering  
Evanston, Illinois 60201

Professor S. B. Dong  
University of California  
Department of Mechanics  
Los Angeles, California 90024

Professor Burt Paul  
University of Pennsylvania  
Towne School of Civil and  
Mechanical Engineering  
Philadelphia, Pennsylvania 19104

Professor H. W. Liu  
Syracuse University  
Department of Chemical Engineering  
and Metallurgy  
Syracuse, New York 13210

Professor S. Bodner  
Technion R&D Foundation  
Haifa, Israel

Professor Werner Goldsmith  
University of California  
Department of Mechanical Engineering  
Berkeley, California 94720

Professor R. S. Rivlin  
Lehigh University  
Center for the Application  
of Mathematics  
Bethlehem, Pennsylvania 18015

Professor F. A. Cozzarelli  
State University of New York at  
Buffalo  
Division of Interdisciplinary Studies  
Karr Parker Engineering Building  
Chemistry Road  
Buffalo, New York 14214

Universities (Con't)

Professor Joseph L. Rose  
Drexel University  
Department of Mechanical Engineering  
and Mechanics  
Philadelphia, Pennsylvania 19104

Professor B. K. Donaldson  
University of Maryland  
Aerospace Engineering Department  
College Park, Maryland 20742

Professor Joseph A. Clark  
Catholic University of America  
Department of Mechanical Engineering  
Washington, D.C. 20064

Dr. Samuel B. Batdorf  
University of California  
School of Engineering  
and Applied Science  
Los Angeles, California 90024

Professor Isaac Fried  
Boston University  
Department of Mathematics  
Boston, Massachusetts 02215

Professor E. Krempl  
Rensselaer Polytechnic Institute  
Division of Engineering  
Engineering Mechanics  
Troy, New York 12181

Dr. Jack R. Vinson  
University of Delaware  
Department of Mechanical and Aerospace  
Engineering and the Center for  
Composite Materials  
Newark, Delaware 19711

Dr. J. Duffy  
Brown University  
Division of Engineering  
Providence, Rhode Island 02912

Dr. J. L. Swedlow  
Carnegie-Mellon University  
Department of Mechanical Engineering  
Pittsburgh, Pennsylvania 15213

Universities (Con't)

Dr. V. K. Varadan  
Ohio State University Research Foundation  
Department of Engineering Mechanics  
Columbus, Ohio 43210

Dr. Z. Hashin  
University of Pennsylvania  
Department of Metallurgy and  
Materials Science  
College of Engineering and  
Applied Science  
Philadelphia, Pennsylvania 19104

Dr. Jackson C. S. Yang  
University of Maryland  
Department of Mechanical Engineering  
College Park, Maryland 20742

Professor T. Y. Chang  
University of Akron  
Department of Civil Engineering  
Akron, Ohio 44325

Professor Charles W. Bert  
University of Oklahoma  
School of Aerospace, Mechanical,  
and Nuclear Engineering  
Norman, Oklahoma 73019

Professor Satya N. Atluri  
Georgia Institute of Technology  
School of Engineering and  
Mechanics  
Atlanta, Georgia 30332

Professor Graham F. Carey  
University of Texas at Austin  
Department of Aerospace Engineering  
and Engineering Mechanics  
Austin, Texas 78712

Dr. S. S. Wang  
University of Illinois  
Department of Theoretical and  
Applied Mechanics  
Urbana, Illinois 61801

Professor J. F. Abel  
Cornell University  
Department of Theoretical  
and Applied Mechanics  
Ithaca, New York 14853

Universities (Con't)

Professor V. H. Neubert  
Pennsylvania State University  
Department of Engineering Science  
and Mechanics  
University Park, Pennsylvania 16802

Professor A. W. Leissa  
Ohio State University  
Department of Engineering Mechanics  
Columbus, Ohio 43212

Professor C. A. Brebbia  
University of California, Irvine  
Department of Civil Engineering  
School of Engineering  
Irvine, California 92717

Dr. George T. Hahn  
Vanderbilt University  
Mechanical Engineering and  
Materials Science  
Nashville, Tennessee 37235

Dean Richard H. Gallagher  
University of Arizona  
College of Engineering  
Tucson, Arizona 85721

Professor E. F. Rybicki  
The University of Tulsa  
Department of Mechanical Engineering  
Tulsa, Oklahoma 74104

Dr. R. Haftka  
Illinois Institute of Technology  
Department of Mechanics and Mechanical  
and Aerospace Engineering  
Chicago, Illinois 60616

Professor J. G. de Oliveira  
Massachusetts Institute of Technology  
Department of Ocean Engineering  
77 Massachusetts Avenue  
Cambridge, Massachusetts 02139

Dr. Bernard W. Shaffer  
Polytechnic Institute of New York  
Route 110  
Farmingdale, New York 11735

474:NP:716:lab  
78u474-619

Industry and Research Institutes

Dr. Norman Hobbs  
Kaman Avidyne  
Division of Kaman  
Sciences Corporation  
Burlington, Massachusetts 01803

Argonne National Laboratory  
Library Services Department  
9700 South Cass Avenue  
Argonne, Illinois 60440

Dr. M. C. Junger  
Cambridge Acoustical Associates  
54 Rindge Avenue Extension  
Cambridge, Massachusetts 02140

Mr. J. H. Torrance  
General Dynamics Corporation  
Electric Boat Division  
Groton, Connecticut 06340

Dr. J. E. Greenspon  
J. G. Engineering Research Associates  
3831 Menlo Drive  
Baltimore, Maryland 21215

Newport News Shipbuilding and  
Dry Dock Company  
Library  
Newport News, Virginia 23607

Dr. W. F. Bozich  
McDonnell Douglas Corporation  
5301 Bolsa Avenue  
Huntington Beach, California 92647

Dr. H. N. Abramson  
Southwest Research Institute  
8500 Culebra Road  
San Antonio, Texas 78284

Dr. R. C. DeHart  
Southwest Research Institute  
8500 Culebra Road  
San Antonio, Texas 78284

Dr. M. L. Baron  
Weidlinger Associates  
110 East 59th Street  
New York, New York 10022

Industry and Research Institutes (Con't)

Dr. T. L. Geers  
Lockheed Missiles and Space Company  
3251 Hanover Street  
Palo Alto, California 94304

Mr. William Caywood  
Applied Physics Laboratory  
Johns Hopkins Road  
Laurel, Maryland 20810

Dr. Robert E. Dunham  
Pacifica Technology  
P.O. Box 148  
Del Mar, California 92014

Dr. M. F. Kanninen  
Battelle Columbus Laboratories  
505 King Avenue  
Columbus, Ohio 43201

Dr. A. A. Hochrein  
Daedalean Associates, Inc.  
Springlake Research Road  
15110 Frederick Road  
Woodbine, Maryland 21797

Dr. James W. Jones  
Swanson Service Corporation  
P.O. Box 5415  
Huntington Beach, California 92646

Dr. Robert E. Nickell  
Applied Science and Technology  
3344 North Torrey Pines Court  
Suite 220  
La Jolla, California 92037

Dr. Kevin Thomas  
Westinghouse Electric Corp.  
Advanced Reactors Division  
P. C. Box 158  
Madison, Pennsylvania 15663

Dr. H. D. Hibbitt  
Hibbitt & Karlsson, Inc.  
132 George M. Cohan Boulevard  
Providence, Rhode Island 02903

Dr. R. D. Mindlin  
89 Deer Hill Drive  
Ridgefield, Connecticut 06877

474:NP:716:lab  
78u474-619

Industry and Research Institutes (Con't)

Dr. Richard E. Dame  
Mega Engineering  
11961 Tech Road  
Silver Spring, Maryland 20904

Mr. G. M. Stanley  
Lockheed Palo Alto Research  
Laboratory  
3251 Hanover Street  
Palo Alto, California 94304

Mr. R. L. Cloud  
Robert L. Cloud Associates, Inc.  
2972 Adeline Street  
Berkeley, California 94703

